

Comparative Analysis of Application of Seismic Wave Reflection Method in Advanced Geological Prediction

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Abstract: Seismic wave reflection method is an advanced geophysical detection method in tunnel geological prediction. It is more sensitive and effective in detecting geological anomalies such as fault fracture zone and karst. In order to verify the prediction efficacy and accuracy of the seismic wave reflection method with different instruments and equipment (tunnel geological prediction [TGP]/tunnel seismic prediction [TSP]) and different vibration modes (hammering, explosives), a comparison test was carried out in Jinping Tunnel. The test results showed that the time-consumption of the hammering source was short, which can greatly reduce the impact on the construction site; different vibration sources methods of seismic wave reflection can predict the unfavorable geological sections accurately.

Keywords: Seismic wave reflection method; Vibration source; TSP; TGP

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1. Introduction

In recent years, highway tunnel construction has developed rapidly. As a concealed project, tunnel construction is complex and unpredictable, and often encounters areas with complex geological conditions, especially tunnel sites passing through hollow areas, fault structural zones, karst developed areas and dangerous areas with high gas concentration often cause geological disasters such as landslides, mud gushes, and gas explosions during construction. Carrying out advanced geological prediction of tunnels can timely detect the location, type, and possible risks of unfavorable geological bodies in front of the tunnel face. It can prevent possible geological disasters such as tunnel collapse, water gushing, mud inrush and gas explosion. At the same time, through advanced geological prediction, it is possible to grasp the geological structure conditions and surrounding rock grade types within a short distance in front of the tunnel face, and provide a more scientific basis for the construction unit to select excavation methods and support parameter types. To sum up, advanced geological prediction of tunnels has significant economic and social benefits as it increases construction efficiency, reduces construction period, ensures safe and scientific construction, and reduces construction accident losses.

There are many advanced geological prediction methods, which can be mainly divided into three categories: traditional geological analysis method, direct drilling method of face drilling and more advanced geophysical detection method ^[1-3]. Geophysical advanced geological prediction methods can be divided into long-range prediction and short-range prediction according to different prediction distances. In long-

distance forecast, the geological conditions at a distance of 100–200 m in front of the tunnel face is forecasted mainly through elastic wave reflection. By analyzing the kinematics and dynamics characteristics of the reflected wave received by the geophone and obtaining the imaging information of the rock mass structure, the geological conditions in front of the tunnel can be predicted ^[4]. According to the observation system layout, data processing method, excitation method, etc. ^[5], seismic wave reflection method can be divided into tunnel seismic prediction (TSP) and tunnel geological prediction (TGP); land sonar two-dimensional advanced prediction such as the negative apparent velocity method and the tunnel seismic wave reflection tomography (TRT), tunnel seismic tomography (TST), horizontal sound probing (HSP), and underground seismic prediction system(USP) ^[6]; tunnel seismic detection system (TSD), and other advanced prediction methods of space observation methods. The seismic wave reflection method has a long prediction distance and has a better prediction effect on planar structures with different mechanical properties. Therefore, it has been widely used in the advance geological prediction of road tunnels ^[7].

2. Principle of seismic wave reflection method

The seismic wave signal generated by hammering or small-dose blasting at a specific position in the tunnel propagates in the form of spherical waves along the direction of the tunnel; the seismic wave propagates at different speeds in different rock formations. Seismic waves are generated at different locations by exciting multiple source points. These source points are distributed at specific locations in the tunnel. When the seismic waves encounter abnormal bodies (broken zones, faults, cavities, and many more) in front of the tunnel, the waves will then be reflected to the sensor. The three-axis high-sensitivity sensor will receive the reflected waves (X, Y, Z) from different directions of the abnormal body, so as to obtain a large number of three-dimensional data sets (**Figure 1**). According to the location of the sensor distribution, the propagation direction of the reflected wave of the abnormal body is different from the angle of the sensor at different positions. By calculating the angle and wave velocity of each reflected wave, we can obtain the three-dimensional space position of the abnormal body.



Figure 1. Schematic diagram of the principle of seismic wave reflection method

3. Comparative analysis of application effect

3.1. Instrument efficacy analysis

G4216 Yanjiang Expressway is the largest single-invested expressway in China. The bridge-tunnel ratio of the Yibin-Jinyang section is as high as 92%, of which the tunnel accounts for 70%. The nature of the tunnel is complex and vulnerable, and the unfavorable geological conditions of the cave body are unfavorable, and it is facing a relatively large construction risk. Therefore, the safety and progress of the Yanjiang high-

speed tunnel project are the key to the construction of the expressway. Advanced geological prediction of the tunnel is the most important means of information construction, which provides the basis for the dynamic design and safe construction of the tunnel. In order to compare the efficacy and accuracy of long-distance prediction by seismic wave reflection method, the Chief Engineering Office of Yanjiang Yijin Company established relevant third-party testing units and instruments to conduct comparison tests in the left tunnel of XJ8 Jinping Tunnel. The main instruments and equipment and observation methods are shown in **Table 1**. Four sets of seismic wave reflection instruments were used for the test, two sets of TGP206G instruments were excited by explosive vibration sources; two sets of TSP (YWZ11-Z/305plus) instruments were excited by a hammering vibration source.

Serial number	Instrument	Observation method	Vibration source	Excitation points	Receiving point	Duration forecast
1	TGP206G-1	Side wall line	Dynamite	24	2	 1 hour for 4 drilling rigs/4 workers to complete the drilling of the blasthole. 5 minutes for 1 blasthole to charge the data. 120 minutes/2 hours for 24 blastholes. Total time spent: approximately 3 hours
2	TGP206G-2	Side wall line	Dynamite	24	2	 1 hour for 4 drilling rigs/4 workers to complete the drilling of the blasthole. 5 minutes for 1 blasthole to charge the data. 120 minutes/2 hours for 24 blastholes Total time spent: approximately 3 hours
3	T SP YWZ11-Z	Side wall line	Hammering	24	2	 Acquisition array layout and instrument connection time is 10 minutes. Acquisition parameter setting and trial acquisition time is 2 minutes. Data acquisition time is 14 minutes. 4 minutes to pack up the instrument and equipment Total time spent: 30 minutes/half an hour

Table 1. Instrument and equipment layout and efficiency comparison table

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Figure 3. On-site detection layout of TSP seismic wave reflection method

tunnel face

The TGP measuring line was laid on the left side wall of the tunnel face, about 1.5m above the ground, and a total of 24 blastholes were arranged 1.5 m away from each other to stimulate P1–P24; two receiving points C1 and C2 were located on the left and right side walls, respectively; the offset distance was 15 m (**Figure 2**), and the prediction distance was 150 m. The TSP measuring line was laid on the left side wall of the tunnel face, about 1.5 m above the ground, with a total of 24 hammering points P1–P24, with a distance of 0.7 m between each point; two receiving points; C1 and C2, with a distance of 1.5 m between each other, and the moving distance was 6 m (**Figure 3**); and the prediction distance was 100 m.

Advance geological prediction by seismic wave reflection method is a comprehensive technical work,

in which the source is an important link, and the signal acquisition quality and detection distance are all restricted by the source ^[8]. At present, the seismic sources used in advanced prediction at home and abroad are mainly divided into two categories: one is the expansion point source, such as explosives and spark sources; the other is the surface impact source, such as hammering, controlled shock source, and many more ^[9]. The maximum energy that can be excited and generated of different sources in order from strong to weak are explosive sources > electric spark sources > vibrators > hammering; in which the order would be reversed in terms of convenience of use. Explosive sources are the most frequently used, and the development and application of many advanced geological prediction technologies based on seismic wave methods are based on them, but they are strictly controlled and have great limitations. The comparison test of the seismic wave reflection method used explosive sources and the hammering sources respectively. Affected by the control of explosives, the source of explosives has been gradually replaced by digital electronic detonators from traditional electric detonators. In the advanced geological prediction of the TGP seismic wave reflection method, it is necessary to scan codes one by one to activate blastholes, which takes a long time. It can be seen from **Table 1** that it takes about 3 hours for the advanced geological prediction of the TGP explosive seismic source from the layout and drilling to the data acquisition, whereas it only takes 30 minutes from the layout of the collection array to the data collection for the advanced geological prediction using TSP hammering source. Therefore, seismic wave reflection method using hammering source is better than explosive source in terms of efficacy.

3.2. Comparative analysis of prediction results

This prediction comparison test was carried out in the left tunnel of XJ8 Jinping Tunnel, the pile number of the tunnel face is ZK41+444, and full-section excavation was carried out. The surrounding rock of the tunnel face was mainly blue-gray limestone, with a gently dipping, nearly horizontal layered, thin-to-medium-thick layered structure, and the weathered surface was light grayish white, mainly moderately weathered. Based on the hammering sound and rebound, the harder rock has more well-developed joints and fissures, which are mainly structural and weathered types, and the width of the fissures is mainly micro-extensive, mostly filled with mud, and the interlayer bonding force and stability are poor; the surrounding rocks were generally broken, showing massive to sub-massive structure. Moreover, the arch and the surrounding rock of the vault were easy to fall off or collapse, underground fissure water had been developed, the tunnel face was wet, and the vault top was sporadically dripping. The grade of the surrounding rock was evaluated comprehensively on site to be grade IV (**Figure 4**).



Figure 4. Photos of the surrounding rock conditions of ZK41+444 tunnel face

TGP206G explosive source and the TSP hammering source were used respectively to carry out advanced geological prediction work on the tunnel face ZK41+444. Through data processing, the main











Figure 7. TSP method reflection horizon and physical and mechanical parameters result map (TSP305 PLUS)

The main forecast conclusions are shown in **Table 2**.

Serial number	Instruments/sources used	Forecast range and length	Forecast conclusion	Unfavorable geological body range
1	TGP206G/Dynamite	ZK41+444– ZK41+544	Grade IV surrounding rock	 There was a weak interface near ZK41+464, ZK41+477–ZK41+487, multiple weak interfaces near ZK41+510–ZK41+528, and a weak interface near ZK41+540. It was speculated that the rock mass in this mileage segment was broken and developed fissures or dissolved cavities. There may be seepage of dissolved water, local strands of water may flow out, poor interlayer bonding.
2	TGP206G/Dynamite	ZK41+444- ZK41+544	Grade IV surrounding rock	 In the mileage section ZK41+455–ZK41+470, there were locally developed dissolved fracture zones, weak interlayers and dissolved pipes on the right middle side, and the cracks and interlayers were mostly filled with mud and sand. In the mileage section ZK41+474–ZK41+480, there were dense cracked broken zones locally on the right-middle side. In the mileage section ZK41+485–ZK41+510, there were locally dissolved broken cracked zones, dissolved pipes, or dissolved cavities developed on the right middle side, and the cracks and interlayers were mostly filled with mud and sand, the groundwater is relatively developed, and the local karst pipeline water is exposed. Therefore, during the excavation process, it was necessary to pay attention to the impact of ZK41+467, ZK41+487, ZK41+499, ZK41+508 anomalies on the tunnel. The ZK41+516–ZK41+528 mileage section had locally developed dissolved cracks and broken zones, and local weak interlayers, it was inferred that in the ZK41+560–ZK41+572 mileage section, there were locally developed dissolved cracks and

Table 2. Statistics table of TSP advanced geological prediction results

(Continued on next page)

Serial	Instruments/sources	Forecast	Forecast	Unfavorable geological body range
number	used	range and	conclusion	
		length		
3	TSP YWZ11-Z	ZK41+444-	Grade IV	• In the vicinity of ZK41+482, ZK41+486,
	/hammering	ZK41+544	surrounding	ZK41+498, and ZK41+508, the joints and
			rock	fissures in the ZK41+519–ZK41+541 section
				were relatively developed, with some areas
				having densely developed joint and fissures or
				interbed with weak interlayers. The
				surrounding rocks were relatively broken, and
				karst was relatively developed. Dissolved
				structures such as dissolved pores, dissolved
				pipes, and caves had developed. Groundwater
				had developed, showing seepage or strands.
4	TSP305PLUS/hammering	ZK41+444-	Grade IV	• ZK41+465–ZK41+471, ZK41+477–
		ZK41+544	surrounding	ZK41+486 sections had strong reflective
			rock	surfaces, and it was speculated that dissolved
				fissures, pipes, or joint fissures had densely
				developed. Groundwater had developed in the
				sections, and strands of water may occur
				nearby. ZK41+494–ZK41+503, ZK41+509–
				ZK41+518, ZK41+526–ZK41+530 may have
				had dense joint fissures or dissolved fissures
				and pipelines.

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Geological sketches of the face of ZK41+444–ZK41+544 were drawn by professional geologists. According to the sketches of the face of the site, the surrounding rocks of ZK41+444–ZK41+544 were mainly limestones. The rocks were mostly hard rocks that were thin to medium-thick, with broken rock mass, well-developed joints and fissures, local mud inclusions in the fissures, and poor bonding between structures, which make them Grade IV. The differences between the sketches were the degree of fragmentation of the surrounding rock and the development of underground fissure water. The situation of the surrounding rocks at section ZK41+444–ZK41+516 was similar. After ZK41+516, the surrounding rock became thin layered, and the overall integrity became poor. At the same time, ZK41+477 began to develop fissure water, at ZK41+497–ZK41+533, it became drizzling water, and there was no crack water after ZK41+533.

Table 3. Comparison table of forecast and excavation of ZK41+444, ZK41+477, ZK41+497, ZK41+516, ZK41+533

Excavation	No obvious	Fissure water	Drizzling water	Drizzling water	Thin
results	geological			with thin	surrounding
	anomalies			surrounding	rocks
				rocks	
1	It was predicted	ZK41+477-	There were	There were	The surrounding
	that there was no	ZK41+487 had	multiple weak	multiple weak	rocks were
	abnormality in	strongly reflective	interfaces in	interfaces in	broken.
	this section.	interface, which	ZK41+510-	ZK41+510-	
		meant that the	ZK41+528, which	ZK41+528, which	
		surrounding rocks	meant that rock	meant that the	
		were broken, and	mass was broken,	rock mass was	
		there might be	and there might	broken, and there	
		water seepage	be strands of	might be strands	
		from the cracks.	water gushing	of water gushing	
			out.	out.	
Consistency	Unanimous	Unanimous	Similar	Similar	Unanimous
analysis					
Accuracy			77%		
2	From ZK41+455	ZK41+474-	In the right	ZK41+516-	The surroundin
	to ZK41+470,	ZK41+480 had	middle part of	ZK41+528	rocks were
	there were locally	developed dense	ZK41+485–	mileage section	broken.
	developed	fracture zone.	ZK41+510, there	had locally	
	fracture zones,		were dissolved	developed	
	weak interlayers,		fracture zones,	dissolved fracture	
	and dissolved		corrosion pipes or	zone, partially	
	pipes on the right		dissolved cavities,	interbedded with	
	middle side, and		most of the	weak interlayers.	
	the gaps and		fissures and		
	interlayers were		interlayers were		
	mostly refilled		refilled with mud		
	with mud and		and sand;		
	sand.		groundwater had		
			developed, and		
			water in local		
			karst pipes was		
			exposed.		
Consistency	Unanimous	Similar	Different	Similar	Unanimous
analysis					
Accuracy		l	80 %		

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Excavation results	No obvious	Fissure water	Drizzling water	Drizzling water with thin	Thin	
results	geological				surrounding	
	anomalies			surrounding rocks	rocks	
3	The hardness of	ZK41+482,	Strong reflection	There were joints	There were joints	
	the surrounding	ZK41+486, ZK41	interface of	and fissures in	and fissures in	
	rock is basically	+498 strong	ZK41+498 and	ZK41+519–	ZK41+519–	
	the same as that	reflection	ZK41+508,	ZK41+541, where	ZK41+541, where	
	of the tunnel face,	interface,	development of	some areas had	some areas had	
	the rock mass is	surrounding rock	dissolved fracture	dense ones.	dense ones.	
	relatively	broken,	zone and			
	complete, the	groundwater	development of			
	joints and fissures	developed	groundwater			
	are well					
	developed, the					
	bonding degree of					
	the structural					
	plane is average,					
	and the stability is					
	poor					
Consistency	Unanimous	Similar	Different	Similar	Unanimous	
analysis						
Accuracy			79%	1	1	
4	The hardness of	ZK41+472-	ZK41+486-	ZK41+503-	The surrounding	
	the surrounding	ZK41+486 had a	ZK41+503 might	ZK41+521 might	rocks were	
	rocks was	strong reflective	have dissolved	have weak	broken.	
	basically the same	surface, it was	fractures, karst,	interlayers or		
	as that of the	speculated that	and well-	dissolved fracture		
	tunnel face, the	there might be a	developed	zones. There		
	rock mass was	dense zone of	groundwater	might be rain		
	relatively	dissolved fissures,		strands of water		
	complete, the	pipes or joint		on the face after		
	joints and fissures	fissures, and		excavation		
	were well	groundwater had				
	developed, the	developed in that				
	bonding between	area				
	the structural					
	planes was					
	average, and the					
	stability is poor					
Consistency	Unanimous	Different	Similar	Similar	Unanimous	
analysis						
Accuracy			75%			

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The comparison between forecast results and excavation results is shown in **Table 3**. The advanced geological prediction by seismic wave reflection method carried out using different sources, instruments, and equipment all predicted the surrounding rock 100 m in front of the tunnel face to be Grade IV, which was basically consistent with the geological conditions of the site excavation. The seismic wave reflection method of different instruments and equipment all predict that the ZK41+516–ZK41+533 section has developed fissure water and the surrounding rock was broken. The accuracy of the prediction was above 70%. The main reasons for the discrepancy with the actual excavation are explained below.

(1) Selection of direct wave velocity

In the seismic wave reflection method, the direct wave is received before the reflected wave. For the initial value picking, different personal understandings and selection points will result in different velocity values. The difference in the selection of the direct wave speed will lead to differences in the final results, which will affect the accuracy of the conclusion.

- (2) Selection of band-pass filter parameters Improper selection of band-pass filters will often cause loss of valuable waveform signals, or selection of clutter interference within 300 ms. This requires experience and understanding of the site, and a summary and analysis of the filter selection methods for different strata and different lithologies in order to make a breakthrough.
- (3) Selection of forecast distance

Assuming that the data collected were valid, with the same initial value and band-pass filter parameters, different forecast distances were selected for the same data, and comparisons and inferences were made through verification and comparison from multiple excavations. As the forecast distance increases, the forecast accuracy rate decreases linearly ^[10]. The empirical forecast distance is roughly 100 m for the geologically complex section; the forecast distance for the normal section is between 120 and 180 m.

(4) The impact of excavation footage

The left hole ZK41+444–ZK41+544 of the Jinping Tunnel in the predicted section underwent full-face excavation, and the single-cycle footage was relatively large, so it was difficult to fully reveal the predicted anomalies such as small, dissolved channels, and there were certain differences between the excavation and forecast results.

4. Conclusion

- (1) The hammering source seismic wave reflection method is superior to the explosive source in terms of efficacy, 2.5 hours can be saved in one forecast, which greatly reduces the impact on construction.
- (2) In this test, the seismic wave reflection method using different instruments and shock modes can predict the subsurface section of ZK41+516 to ZK41+533 in the left tunnel of Jinping Tunnel more accurately, where underground fissure water had developed and the surrounding rock was broken. The seismic wave reflection method is sensitive to geological anomalies with differences in elastic wave impedance and has a good detection effect on unfavorable geological factors that affect the integrity of surrounding rocks (joint fissure development, faults and fracture zones, karst, alteration, and many more).
- (3) Although the hammering source is a widely used non-explosive source, its characteristics of weak energy, instability, uncontrollability and poor anti-interference ability should also be considered.
- (4) Affected by the subjective factors of the geological description of the tunnel face, the excavation of the surrounding rock was not very accurate, and there were no major adverse geological phenomena in the forecast section. Therefore, more research and improvements need to be done. On one hand, professional geologists need to conduct macroscopic geological understanding analysis; on the other hand, it is also necessary for geophysical exploration personnel with theoretical knowledge and field

experience to carry out geophysical exploration interpretation, eliminate interference anomalies and multiple solutions, and carry out advanced geological prediction truthfully and objectively.

(5) The single geophysical prospecting method utilizes the characteristics of geological body for advanced geological prediction. For example, the seismic wave reflection method uses the difference in wave impedance of the surrounding rock; the geological radar uses the difference in the dielectric constant of the surrounding rock; and the transient electromagnetic method uses difference in the surrounding rock. These methods have their limitations with solutions to them. The comprehensive overdue geological prediction method of geological analysis, geophysical prospecting, and drilling should be used for prediction to improve the accuracy of the forecast and ensure safe construction of the tunnel project.

Disclosure statement

The authors declare no conflict of interest.

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